SAFETY AND SERVICEABILITY

Evaluation and Epoxy-Injection Repair of Cracks in Concrete

by Leo Mancs, Vector Construction

The addition of steel reinforcement to concrete in the 19th century enabled structural concrete elements to work not only in compression but also in tension, which caused cracking in tension areas. Other common causes of concrete cracking are plastic shrinkage, settlement, drying shrinkage, thermal stresses, chemical reaction (due to alkali-reactive silica or sulfates), freezing and thawing, corrosion of reinforcing steel, overload stresses, poor construction practices, and design or detailing errors.

Even if concrete cracking does not affect structural performance, it can adversely affect the durability of structures by allowing chloride or carbon dioxide deep into the structural element and initiating corrosion over time. To some extent, cracking can be minimized or controlled by employing appropriate design, detailing, and construction practices and by incorporating advanced construction materials into new concrete structures.

ACI 224R-01: Table 4.1 provides a general guideline for reasonable crack widths for reinforced concrete under service loads for new construction. Acceptable crack widths range from 0.004 in. (0.1 mm) to 0.016 in. (0.41 mm), with smaller widths for concrete in wet or aggressive environments and larger crack widths for drier exposures. It is up to the specifier to use sound engineering judgment along with applicable standards to determine crack widths that may lead to a loss in functionality of the concrete structure.

For a concrete structure, once the allowable crack width is established and cracks are identified, their widths and depths and the cause of cracking should be determined. The width of cracks can be measured to a precision of 0.001 in. (0.025 mm) by using crack gauge cards or pocket microscopes. It is important that the actual width of the crack is measured at a clean and straight location along the crack and that the measurement does not include the width of chipped or worn edges of the crack. To determine the depth and cause of cracking, core samples can be taken and, if subjected to petrographic testing, analyzed for issues such as freeze-thaw resistance, alkali-silica reactivity, or delayed ettringite formation. The depth of cracking can also be evaluated by a nondestructive testing (NDT) method such as ultrasonic pulse velocity.

Because the bond strength of epoxy to concrete is greater than the tensile strength of concrete, epoxy injection can restore the structural integrity of the concrete when applied to cracks 0.002 in. (0.05 mm) in width or greater. It is important to note that if the cause of cracking is corrosion of the reinforcing steel or if movement of the concrete is anticipated after repairs, epoxy injection may not be the best solution to the problem.

Shallow cracks on horizontal surfaces such as bridge decks or the tops of pier caps can be repaired by gravity feed of epoxy. The face of the crack is temporarily coated with nonpressure pipes that are incised around the ports before the injection process begins.

Crack widths can be visually measured using a gauge card (middle) or pocket microscopes. All Photos: Vector Construction.

Table 4.1 Guide to reasonable* crack widths, reinforced concrete under service loads

<table>
<thead>
<tr>
<th>Exposure condition</th>
<th>Crack width</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>in.</td>
</tr>
<tr>
<td>Dry air or protective membrane</td>
<td>0.016</td>
</tr>
<tr>
<td>Humidity, moist air, soil</td>
<td>0.012</td>
</tr>
<tr>
<td>Deicing chemicals</td>
<td>0.007</td>
</tr>
<tr>
<td>Seawater and seawater spray, wetting and drying</td>
<td>0.006</td>
</tr>
<tr>
<td>Water-retaining structures†</td>
<td>0.004</td>
</tr>
</tbody>
</table>

*It should be expected that a portion of the cracks in the structure will exceed these values. With time, a significant portion can exceed these values. These are general guidelines for design to be used in conjunction with sound engineering judgement [sic].
†Excluding [sic] nonpressure pipes.
Note: Table 4.1, Guide to Reasonable Crack Widths, Reinforced Concrete Under Service Loads, was reproduced from ACI 224R-01 (Reapproved 2008) Control of Cracking of Concrete Structures with permission from the American Concrete Institute.

After the surface has been cleaned, surface-mounted injection ports are installed. A temporary crack-sealer epoxy paste will be applied on the face of the crack between and around the ports before the injection process begins.
For consistent performance on large or critical projects, use of a dual-component pump with positive displacement and metering capabilities is recommended. After calibration, the epoxy materials are metered by the pump and mixed to the proper ratio in a static mixer just before entering the crack.

Typically, wire brushing and vacuuming the crack surface on a 2-in.-wide (50-mm-wide) strip along the crack is sufficient for surface preparation. In some cases, pressure washing, grinding, or V-grooving may be needed to access clogged cracks.

Surface-mounted injection ports are adequate for injecting most cracks. Where the surface of the crack is blocked or the width of the cracks is \( \frac{1}{8} \) in. (3 mm) or larger, plastic tubing ports are directly inserted in open cracks or sealed in holes drilled to intersect the cracks. The injection ports are typically installed at a spacing that is equal to or greater than the measured depth of the crack.

Once the injection ports are installed, a temporary crack-sealer epoxy paste is applied on the face of the crack between and around the ports. To contain the epoxy resin in the crack until it hardens, it is a good practice to seal cracks on all sides of the concrete element.

Injection of epoxy always begins at the lowest elevation. Once the epoxy emerges at the next higher port, the current port is capped and the injection is continued from the port where the epoxy emerged. The process continues in this manner until all ports are injected. Typical injection pressures are between 50 to 100 psi (350 to 700 kPa), with very fine cracks requiring pressures of 200 psi (1.4 MPa) or higher, provided that the crack seal is not damaged by the high pressure. Raising ambient and resin temperatures can lower resin viscosity and require less pressure. Minor epoxy leaks through hairline cracks can be sealed using paraffin wax. Once the crack sealant has hardened sufficiently, the crack-sealer epoxy paste is usually removed from the surface of the concrete according to manufacturer’s recommendations.

Quality assurance/control measures to ensure that cracks are sufficiently repaired may include visual observation of the injection process, laboratory testing of mixed epoxy, testing the injection equipment for mix ratio under pressure, evaluation of cores sampled through the injected crack, and testing across the repaired cracks using NDT methods such as ultrasonic pulse velocity, impact echo, or spectral analysis of surface waves.7

Specialist concrete repair contractors with experience in the epoxy-injection process and manufacturers of injection resins and equipment are valuable resources for providing means, methods, and training for epoxy injection.

References
2. ACI. 2008. Control of Cracking of Concrete Structures (ACI 224R-01). Farmington Hills, MI: ACI.

Leo Manes is division manager, technical services and special projects, at Vector Construction in Winnipeg, Manitoba.